#### **INDIAN INSTITUTE OF TECHNOLOGY ROORKEE**



### **Evaluation of Safety Performance of Road Infrastructure**

### by Surrogate Measures





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### **Public Profiles**





# **Academic Qualifications**

- B.E. in Civil Engineering 2003
  - ✓ Indian Institute of Engineering Science and Technology (IIEST), Shibpur, West Bengal
     ✓ Cleared GATE 2003 (All India Rank 566)
- M.E. in Civil Engineering (Spl: Highway and Traffic Engineering) 2005
   ✓ Indian Institute of Engineering Science and Technology (IIEST), Shibpur, West Bengal
- M.Sc. in Civil Engineering (Spl.: Transportation Engineering) 2008
   ✓ Wayne State University, Detroit, Michigan, USA
- Ph.D. in Civil Engineering (Spl.: Transportation Engineering) 2010
   ✓ Wayne State University, Detroit, Michigan, USA
- Post-Doc @IIT Madras 2011

JOINED IIT Roorkee in July 2012



# **Recognition from Professional Bodies/Awards**

- DUO-India Fellowship (Professor) 2020 to serve as Academic Visitor at Newcastle University, United Kingdom in 2022) – MHRD & ASEM DUO
- International Standing Committee Member, Traffic Control Devices Committee (ACP55), Transportation Research Board (TRB), National Academies, Washington D.C., USA; 2019 - To date
- Member, H-7 Committee: Road Safety and Design, Indian Roads Congress (IRC); 2021 To date
- Chairperson, TCT-101 Committee: Other transportation modes (including NMT) and pedestrian, Transportation Research Group (TRG) of India; 2022 – To date
- Outstanding Teacher Award @IIT Roorkee 2021
- Editorial Board Member, Journal of Advanced Transportation, Wiley, United Kingdom; 2020 To date
- Education-Scholarship Award from Institute of Transportation Engineer (ITE), USA; 2008
- Solo recipient of Thomas C Rumble Fellowship (\$52,000/annum) during PhD; 2007

# Background



Road safety evaluation methodology can be broadly classified into two sub-groups:



### **Surrogate Safety Measures**

#### **Surrogate Safety Measures**

#### FHWA (1989)

# an event that is proximate to and usually preceding an accident

- Volume
- Speed
- Accepted gap
- Headways
- Deceleration

#### **Conflicts (General Motors, 1967)**

A **traffic conflict** is a situation in road traffic where two or more road users (such as vehicles, pedestrians, or cyclists) approach each other in such a way that there is a **risk of a collision** if their movements continue **unchanged**.



Pyramid of Traffic Events (Hyden, 1987)





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# **Traffic Conflict Techniques**





The conflict severity is based on two indicators:

- **Time-to-Accident (TA)** time remaining to a collision when the evasive action is taken by the relevant road user;
- **Conflicting Speed (CS)** speed of the relevant road user when he/she takes the evasive action.

Pedestrian-vehicle conflict studies @IIT Roorkee

# Analysis of Pedestrian Conflict with Right-Turning Vehicles at Signalized Intersections

Pedestrian Deaths: 32,825 in 2022 vs. 15,746 in 2016 (108% increase)

## Mechanism Of Right-turning Vehicle and Pedestrian Conflict



Kumar, A., Paul, M., & Ghosh, I. (2019). Analysis of Pedestrian Conflict with Right-Turning Vehicles at Signalized Intersections in India. Journal of Transportation Engineering, Part A: Systems, ASCE, 145(6), 04019018. https://doi.org/10.1061/JTEPBS.0000239

## Photos from two sites







## **Evaluation of Conflict Indicators**



## **Descriptive Statistics**

Table 1: Minimum, maximum and average values of PET, TTV and DST							
		Site 1	Site 2	Site 3	Site 4		
	Minimum	0.48	0.96	0.84	0.50		
PET values (sec)	Maximum	5.40	4.78	3.21	3.10		
18	Average	2.65	2.41	2.21	2.52		
55	Minimum	0.64	0.68	0.61	0.52		
TTV values (sec)	Maximum	4.51	3.28	2.84	3.00		
	Average	5.16	5.02	3.44	4.91		
	Minimum	0.60	0.54	0.62	0.49		
DST <sub>ped</sub> values (m/sec <sup>2</sup> )	Maximum	2.74	1.85	1.64	1.52		
	Average	1.69	1.24	1.15	1.02		
15	Minimum	0.68	0.62	0.54	1.32		
TTA values (sec)	Maximum	3.24	2.35	1.64	1.52		
	Average	1.32	1.04	0.98	1.14		
	Minimum	0.71	0.50	0.58	0.49		
DST <sub>veh</sub> values (m/sec <sup>2</sup> )	Maximum	1.30	1.85	1.24	1.68		
	Average	1.04	0.98	0.72	1.20		

Kumar, A., Paul, M., & **Ghosh, I.** (2019). Analysis of Pedestrian Conflict with Right-Turning Vehicles at Signalized Intersections in India. *Journal of Transportation Engineering, Part A:* Systems, ASCE, 145(6), 04019018. https://doi.org/10.1061/JTEPBS.0000239

## **Classification of Conflict Indicators by K-mean clustering**



Systems, ASCE, 145(6), 04019018. https://doi.org/10.1061/JTEPBS.0000239



## **Classification of Conflict Indicators by K-mean Clustering**

	Table 5.1 mai claster ranges for indicators							
		Severity level						
Indicators	1	2	3	4				
PET	0.48-2.32	2.40-2.88	3.12-3.56	4.48-5.40				
TTV	0.52-1.12	1.28-1.90	2.42-2.88	3.16-4.51				
DST <sub>ped</sub>	1.50-2.74	1.28-1.44	0.94-1.24	0.48-0.90				
TTA	0.54-0.88	0.94-1.62	1.70-2.26	2.40-3.24				
DST <sub>veh</sub>	1.24-1.68	1.00-1.18	0.58-0.94	0.36-0.52				

Table 3. Final cluster ranges for indicators

1 = Most severe 4 = Least severe

#### Table 4: ANOVA results for indicators

	Clust	er	Erro	r			
Indicators	Mean Square	df	Mean Square	df	F	Sig.	
PET	8.852	3	0.244	130	135.022	0	
TTV	144.226	3	1.980	130	136.525	0.003	
DST <sub>ped</sub>	1.329	3	0.078	130	9.228	0.001	
TTA	12.236	3	1.828	130	88.242	0.000	
DST <sub>veh</sub>	98.122	3	4.560	130	32.446	0.002	

Kumar, A., Paul, M., & Ghosh, I. (2019). Analysis of Pedestrian Conflict with Right-Turning Vehicles at Signalized Intersections in India. Journal of Transportation Engineering, Part A: Systems, ASCE, 145(6), 04019018. https://doi.org/10.1061/JTEPBS.0000239

## **Distribution of Conflicts in Green Interval of Right Turning Traffic**





#### Table 5: Distribution of conflicts with severity level-1

Green Interval	Percentage of conflict with near side pedestrians	Percentage of conflict with far side pedestrians
First quarter	10%	18%
Second quarter	14%	16%
Third quarter	30%	25%
Fourth quarter	46%	51%

# Distribution of conflicts in the green interval of right turning traffic

Kumar, A., Paul, M., & Ghosh, I. (2019). Analysis of Pedestrian Conflict with Right-Turning Vehicles at Signalized Intersections in India. Journal of Transportation Engineering, Part A: Systems, ASCE, 145(6), 04019018. https://doi.org/10.1061/JTEPBS.0000239

# **Relation of Conflicts with Vehicle Type**





#### Frequency of conflicts with different vehicle types

with associated vehicle types at each site

Kumar, A., Paul, M., & Ghosh, I. (2019). Analysis of Pedestrian Conflict with Right-Turning Vehicles at Signalized Intersections in India. Journal of Transportation Engineering, Part A: Systems, ASCE, 145(6), 04019018. https://doi.org/10.1061/JTEPBS.0000239



# **Statistical Modelling Results**

#### Table 6: Results of Logistic Regression Model - Risk taking behavior ["1" (risk taken) and "0" (risk not taken)]

S. No.	Variables	Coefficient	Significance level	Standard Error
1.	Pedestrian with more than one company	-2.189	0.036	0.110
2.	(a) 1.07 m/sec< Crossing speed ≤1.49 m/sec	1.333	0.008	0.215
	(b) Crossing speed >1.49 m/sec	2.352	0.012	0.350
3.	(a) 64 sec $<$ Waiting time $\leq$ 90 sec	1.500	0.045	0.289
	(b) Waiting time >90 sec	3.522	0.049	0.680
4.	Young pedestrians	2.456	0.001	0.442
5.	(a) Male pedestrians	4.318	0.026	1.986
	(b) Female pedestrians	-1.233	0.029	0.200
6.	Rolling while crossing	0.564	0.046	0.018
7.	Third quarter of green interval	4.676	0.002	2.348
	Fourth quarter of green interval	1.912	0.008	0.389
8.	Right turning vehicle volume	0.889	0.010	0.036
9.	Constant	7.864	0.048	4.654

A test of the full model against a constant only model was significant, therefore indicating that the predictors as a set reliably distinguished between 'risk taken' and 'risk not taken' (*chi-square = 42.148, p<0.001 and df =4*).

Nagelkerke's R<sup>2</sup> value of 0.712 indicated a strong relationship between prediction and grouping. Prediction success overall was 81%.

Kumar, A., Paul, M., & **Ghosh, I.** (2019). Analysis of Pedestrian Conflict with Right-Turning Vehicles at Signalized Intersections in India. *Journal of Transportation Engineering, Part A:* Systems, ASCE, 145(6), 04019018. https://doi.org/10.1061/JTEPBS.0000239

# **Summary of Findings**



- ➢ Traffic Conflict Technique: PET, TTV, DST<sub>ped</sub>, TTA and DST<sub>veh</sub>.
- ➢ Four classes of severity levels- 1, 2, 3 & 4
- Maximum conflicts @first and second quarters of green interval.
- Severe most conflicts occurred in the third and fourth quarters of green interval.
- Maximum and severe most conflicts occurred with LMVs such as cars and SUVs.
- Pedestrians' age, gender, waiting time, speed, type of crossing, group, and vehicle volume have a significant effect on the risk-taking behavior of pedestrians.

# **Changes** in Traffic Signal Operational Modes





Analysis of Spontaneous Order of Pedestrian-Vehicle Conflicts at Signalized Intersections

# **Spontaneous Order**



# **<u>Traditional Safety Design</u>**

- Segregation of transportation modes from each other.
- > Focus is to minimize conflicts.
- Use of signs, markings, signals, traffic calming measures, etc.

# Concept of Shared Space

- Removing physical barriers and abolishing rules.
- Behavior of road users to rely on social cues (i.e., spontaneous order).
- No use of signs, signals, and markings, etc.

The disorderly type of setting, built from the silent cues of other road users instead of pre-defined rules, is termed as spontaneous order.

## **Spontaneous Order**







Rinkonomics: A Window on Spontaneous Order

Daniel B. Klein\*

"An important quality of collision is *mutuality*. If I collide with you, then you collide with me. And if I don't collide with you, you don't collide with me. In promoting my interest in avoiding collision with you, I also promote your interest in avoiding collision with me."

#### Daniel B. Klein

#### Ice Skating rink

# **Spontaneous Order**





Skvallertorget, Norrköping, Sweden before and after remodelling of the intersection (Ref: Hamilton-Baillie B, 2008)

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# Study Sites (Zebra Crossing ~ Partial Shared Space)

A total of twelve signalized intersections were selected from the following three cities of India: (1) New Delhi, (2) Jaipur, and (3) Dehradun

Site No.	Name of Sites	Subject Crosswalk	Size of Intersections	Signal Cycle Length (sec)	Conflicting Traffic Flow (veh/hr)	Pedestrian Flow (ped/hr)
Site 1	Mulchand Intersection, New Delhi	Northbound	50m x40m	180	1120	380
Site 2	Peeragarhi Intersection, New Delhi	Westbound	45m x30m	180	1055	462
Site 3	Subhash Nagar Intersection, New Delhi	Eastbound	30m x30m	185	1385	375
Site 4	Madhuban Intersection, New Delhi	Northbound	40m x30m	180	1246	323
Site 5	Darshanlal Intersection, Dehradun	Southbound	20m x18m	140	1150	310
Site 6	Ballupur Intersection, Dehradun	Eastbound	25m x20m	145	1421	282

#### Table 7: Features of the selected intersections



## **Study Sites**

#### Table 7 continued....

Site No.	Name of Sites	Subject Crosswalk	Size of Intersections	Signal Cycle Length (sec)	Conflicting Traffic Flow (veh/hr)	Pedestrian Flow (ped/hr)
Site 7	Crossroad Mall Intersection, Dehradun	Eastbound	20m x18m	135	1210	356
Site 8	Tehsil Intersection, Dehradun	Southbound	20m x18m	140	1100	392
Site 9	JDA Circle, Jaipur	Westbound	25m x22m	145	1350	410
Site 10	Sanganeri Gate Intersection, Jaipur	Westbound	25m x20m	140	1400	322
Site 11	Gopalpura Intersection, Jaipur	Northbound	20m x18m	140	1236	374
Site 12	Shaheed Smarak Intersection, Jaipur	Northbound	40m x20m	145	1108	385



### **Yielding Behavior of Pedestrians and Vehicles**



Normal Dominance: The vehicle continues moving and pedestrian yields to the vehicle by slowing down and allows the vehicle to pass. Coercive Dominance: Vehicle speeds up or accelerates (sometimes blinks the headlight and blows the horn) as an indication to pedestrian to slow down or stop and passes prior to the pedestrian.



#### Pedestrian Dominance

**Normal Dominance:** Pedestrian continues walking, and the vehicle yields to pedestrians by slowing down and allows the pedestrian to pass. **Coercive Dominance:** Pedestrian uses hand gestures by raising the hand and showing palm towards the oncoming vehicles as an indication to slow down or stop, thereby passes prior to the vehicle.



 $Modal Dominance Index (MDI) = \frac{Number of times pedestrians yielded - Number of times vehicles yielded}{Total number of conflicts}$ 

## **Pedestrian-Vehicle Interactions and Conflicts**







	Number of conflicts						
	Entering vehicles			De			
Site No.	Left turn (Type 1)	Straight (Type 2)	Right turn (Type 3)	Left turn (Type 4)	Straight (Type 5)	Right turn (Type 6)	Total
1	21	37	18	20	40	15	151
2	30	42	21	24	48	23	188
3	17	34	20	18	42	17	148
4	15	30	24	16	39	18	142
5	22	15	23	31	22	15	128
6	22	15	16	28	25	14	120
7	18	41	20	21	30	21	150
8	22	35	18	25	37	17	154
9	38	25	20	36	26	21	166
10	19	24	23	31	15	24	136
11	25	38	17	22	35	18	155
12	33	18	20	22	38	16	147
	Total number of conflicts						

#### **Table 8: Frequency of Conflicts**

## **Pedestrian Dominance at Intersection**



#### **Table 9: Proportion of Pedestrians Exhibiting Normal and Coercive Dominance**

	Conflict with Er	ntering Vehicles	Conflict with De	parting Vehicles		
	Near Side Pedestrians	Far Side Pedestrians	Near Side Pedestrians	Far Side Pedestrians		
Normal Dominance	89 (35%)	202 (92%)	124 (46%)	195 (84%)	Entering approach	Exiting approach
Coercive Dominance	164 (65%)	16 (8%)	146 (54 %)	37 (16%)	Neur Side	
Total	253	218	270	232	Pedestrian Pedestrian	Far Side Pedestrian
		1	EC .			•

There was a statistically significant difference ( $\chi^2$ =163.87, p<0.001) in the number of pedestrians of the near side and far side for showing normal and coercive dominance at the intersection for entering vehicles ( $\chi^2$ =163.87, p<0.001) and departing vehicles ( $\chi^2$ =78.29, p<0.001).

## **Analysis of Modal Dominance Index**



#### MDI values for entering vehicles at each intersection





# **Analysis of Modal Dominance Index**



### • MDI values for departing vehicles at each intersection



## **Analysis of Modal Dominance Index**



MDI values for different categories of vehicles







Table 10: Number of yields by pedestrians and vehicles for the types of violations

Violation Type	Number of	Number of Times	Number of
	(Percentage)	Vielded	Vielded
	(reicentage)	Tielded	Tielded
Non-Violation	719 (40.28%)	200	519
Spatial Violation	188 (10.53%)	182	6
Temporal Violation	763 (42.74%)	605	158
Spatial & Temporal Violation	115 (6.45%)	75	40

## **Pedestrian Violation Analysis**

#### MDI values for type of violation



Kumar, A., & Ghosh, I. (2020). Analysis of Spontaneous Order of Pedestrian–Vehicle Conflicts at Signalized Intersections. *Transportation Research Record*, 2674(11), 440-454. https://doi.org/10.1177/0361198120945994





## **Estimated Results of Development of Dominance Model**

Table 11: Estimated results of the binary logit model –
'Vehicle dominance' (1) vs. 'Pedestrian dominance' (0)

Variables	Coefficient	Significance	Standard Error
Vehicles/cycle	4.318	0.002	0.110
Type of vehicle			
2W	-1.254	0.005	1.325
Car	2.535	0.000	0.215
Pedestrian violation			
Temporal violation	4.292	0.011	0.552
Group size of crossing pedestrians			
Single pedestrian	3.262	0.003	0.350
More than two pedestrians	-1.010	0.049	0.289
Type of pedestrian-vehicle interaction			
With left turn entering vehicle	-1.824	0.045	0.680
With straight entering vehicle	2.815	0.026	1.986
Size of intersection			
Small	-5.322	0.001	0.565
Constant	3.875	0.002	1.442

Nagelkerke's R<sup>2</sup> value was found to be 0.814, which indicates a strong relationship between grouping and prediction.

The overall predictive accuracy was 87.7%.



- Modal Dominance Index: *Pedestrian Dominance* & *Vehicle Dominance*
- MDI values for the interaction of pedestrians with straight and turning vehicles needs to be checked.
- Motorized two-wheelers are more likely to yield to pedestrians during the crossing.
- ➢High pedestrian flow leads to a greater number of conflicts at the intersections.
- > The size of the intersection is found to be related to conflict proximity (PET).
- ➢Vehicle volume (per cycle), type of vehicles, pedestrian violation, the group size of crossing pedestrian, type of pedestrian-vehicle interaction, and size of intersection have been found to be in relationship with the dominance of road users at the intersections.

Pedestrian Behavior and the Associated Conflicts at Signalized Intersections

# **Types of Signal Violations**





Violation Prevalence = Number of pedestrians crossing on red Total number of pedestrians that arrived during red phase

## **Pedestrian-Vehicle Conflicts**



#### A conflict is said to have occurred if either of the following interaction types is observed:



3

4

5

A pedestrian yields by slowing down and allows the vehicle to pass.



- A vehicle yields by braking and slowing down and allows the pedestrian to pass.
- A vehicle accelerates and passes the conflict zone before the pedestrian arrives.

Neither of the road users takes evasive action, but there is a small-time lapse between the moments they pass the conflict zone.



# **Study Sites ~ Issue with Pedestrian clearance time**

A total of twelve signalized intersections were selected from the following three cities of India: (1) New Delhi, (2) Jaipur, and (3) Dehradun

#### Table 12: Features of the selected intersections

Site No.	Name of Sites	Subject Crosswalk	Size of Intersections (mxm)	Signal Cycle Length (sec)	Conflicting Traffic Flow (veh/hr)	Pedestrian Flow (ped/hr)
Site 1	Mulchand Intersection, New Delhi	Northbound	50 x40	180	1120	380
Site 2	Peeragarhi Intersection, New Delhi	Westbound	45 x30	180	1055	462
Site 3	Subhash Nagar Intersection, New Delhi	Eastbound	30 x30	185	1385	375
Site 4	Madhuban Intersection, New Delhi	Northbound	40 x30	180	1246	323
Site 5	Darshanlal Intersection, Dehradun	Southbound	20 x18	140	1150	310
Site 6	Ballupur Intersection, Dehradun	Eastbound	25 x20	145	1421	282



# **Study Sites ~ Issue with Pedestrian clearance time**

#### Table 12 continued....

Site No.	Name of Sites	Subject Crosswalk	Size of Intersections (mxm)	Signal Cycle Length (sec)	Conflicting Traffic Flow (veh/hr)	Pedestrian Flow (ped/hr)
Site 7	Crossroad Mall Intersection, Dehradun	Eastbound	20 x18	135	1210	356
Site 8	Tehsil Intersection, Dehradun	Southbound	20 x18	140	1100	392
Site 9	JDA Circle, Jaipur	Westbound	25 x22	145	1350	410
Site 10	Sanganeri Gate Intersection, Jaipur	Westbound	25 x20	140	1400	322
Site 11	Gopalpura Intersection, Jaipur	Northbound	20 x18	140	1236	374
Site 12	Shaheed Smarak Intersection, Jaipur	Northbound	40 x20	145	1108	385

## **Data Description**



#### Table 13: Observed frequencies of violation and non-violation by pedestrians

	Violation	Non-violation	Total
Gender			
Male	1550 (66%)	3554 (62%)	5104
Female	792 (34%)	2193 (38%)	2985
Total	2342	5747	8089
Age			
Young	1439 (61.5%)	1906 (33.2%)	3345
Middle-aged	856 (36.5%)	3108 (54%)	3964
Old	47 (2%)	733 (12.8%)	780
Total	2342	5747	8089

## **Data Description**



		•				
	Number of pedestrians arrived in red		Number of pedestrians arrived in green			
Site No.	Crossed in red	Waited for green	Total	Completed crossing in green	Crossed at the end of green	Total
Site 1	462 (46.9%)	523 (53.1%)	985	510 (95.3%)	25 (4.7%)	535
Site 2	368 (49.1%)	382 (50.9%)	750	1076 (98.0%)	22 (2.0%)	1098
Site 3	292 (45.6%)	348 (54.4%)	640	844 (98.1%)	16 (1.9%)	860
Site 4	244 (40.3%)	361 (59.7%)	605	677 (98.5%)	10 (1.5%)	687
Site 5	225 (38.8%)	355 (61.2%)	580	624 (94.5%)	36 (5.5%)	660
Site 6	185 (38.9%)	290 (61.1%)	475	636 (97.4%)	17 (2.6%)	653
Site 7	258 (41.5%)	363 (58.5%)	621	778 (96.9%)	25 (3.1%)	803
Site 8	329 (46.7%)	376 (53.3%)	705	836 (96.9%)	27 (3.1%)	863
Site 9	362 (47.9%)	394 (52.1%)	756	866 (98.0%)	18 (2.0%)	884
Site 10	240 (40.7%)	349 (59.3%)	589	685 (98.0%)	14 (2.0%)	699
Site 11	310 (45.1%)	378 (54.9%)	688	786 (97.3%)	22 (2.7%)	808
Site 12	333 (47.9%)	362 (52.1%)	695	810 (95.9%)	35 (4.1%)	845
Total	3608	4481	8089	9128	267	9395

#### Table 14: Observed frequencies of pedestrian arrival in red and green



### • Frequency distribution of pedestrian violations







### <u>Violation Prevalence Model</u>

Variable	Standard Error	Coefficient	p-value
Pedestrian volume during red phase	0.020	-5.529	0.001
Vehicle volume during red phase	0.521	-1.334	0.020
Speed of vehicles (km/h)	0.369	-0.512	0.082
Speed of pedestrians (m/sec)	0.045	1.201	0.099
Mean waiting time of pedestrians	0.712	2.821	0.001
during red interval			
Crosswalk length (m)	0.185	-2.538	0.003
R <sup>2</sup> = 0.954, Number	of observations=	950	

#### **Table 15: Model Results for Violation Prevalence**

### • Distribution of Conflicts in Red Phase





- 1. A pedestrian yields by slowing down and allows the vehicle to pass.
- 2. A pedestrian speeds up and passes the conflict zone before the arrival of the vehicle.
- 3. A vehicle yields by braking and slowing down and allows the pedestrian to pass.
- 4. A vehicle accelerates and passes the conflict zone before the pedestrian arrives.
- 5. Neither of the road users takes evasive action, but there is a small-time lapse between the moments they pass the conflict zone.





### <u>Temporal proximity of conflicts</u>



- At the beginning of the red phase, approximately half of the conflicts (about 52%) have PET values of less than 2 seconds.
- At the end of the red interval, about 70% of the conflicts have PET values less than 2 seconds

Emerginning of Red End of Red End of Red ----- Beginning of Red ----- Mid of Red ----- End of Red

#### Comparison of PET distributions in the beginning, middle, and end of the red interval



### • <u>Severity of pedestrian-vehicle conflicts using STCT</u>



Classification of vehicle-pedestrian conflicts based on severity levels



### • <u>Severity of pedestrian-vehicle conflicts using STCT</u>



- About 60% of the serious conflicts occurred at end of red.
- The potential conflicts were predominant (65%) in the beginning of the red interval.
- About 58% of the slight conflicts occurred at the end of the red interval.

### Classification of vehicle-pedestrian conflicts based on severity levels



# Ordered response model for conflict severity

#### Table 16: Estimated results of the ordered logit model

Variable	Standard Error	Coefficient (B)	Exp (B)	p-value
Vehicle speed	0.218	0.981	2.667	0.001
Type of vehicle	A.			
2W	0.214	0.348	1.416	0.010
Gender of pedestrian				
Male	0.308	0.512	1.668	0.004
Group size of crossing pedestrians	1			
Single pedestrian	0.089	0.243	1.275	0.003
Proportion of red phase				
Middle	0.075	0.588	1.800	0.001
End	0.152	0.481	1.617	0.000
Length of crosswalk	54.4			
Large	0.108	0.712	2.038	0.003
Threshold 1	0.402	1.413	3.980	0.855
Threshold 2	0.359	1.205	3.412	1.200
Threshold 3	0.510	-0.452	1.748	0.685

Dependent Variable:

1 = severity level-I (potential conflicts)

2 = severity level-II (slight conflicts)

3 = severity level-III (serious conflicts)

Log-likelihood = -3002.144
Number of observations = 1787
LR chi square (8) = 42.114
Prob > chi square = 0.000
Pseudo $R^2 = 0.598$



# **Summary of Findings**

- 44.6% of pedestrians violate the traffic signals.
- Latter half of the red phase is the violation-dense period.
- Violation prevalence increased with the increase in waiting time of pedestrians.
- The violation prevalence decreased with the increase in pedestrian volume, vehicle volume, and crosswalk length.
- Critical conflicts occurred more frequently at the beginning and end of the red phase.
- Severity of conflicts- Time to accident and conflicting speed.
- Vehicle speed, vehicle type, pedestrians' gender, the group size of crossing pedestrians, proportion of red phase, and crosswalk length were found to be significantly affecting the severity of conflicts.

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# Recent works at IIT Roorkee

Machine learning framework for the development of an interactive pedestrian crossing to enhance pedestrian safety



Studies	Models used	Prediction Horizon	Predictive Performance
Muduli & Ghosh ( 2023)	Random forest (RF)	4s	0.883 (AUC)
Rasouli et al. (2020)	stacked GRU	25	0.829 (AUC)
Kotseruba et al. (2020)	RNN	25	0.85 (AUC)
Muduli & Ghosh ( 2023)	Random forest (RF)	25	0.911 (AUC)
Zhang et al. (2020)	LSTM	1.55	0.938 (AUC)
Muduli & Ghosh ( 2023)	Random forest (RF)	1.55	0.940 (AUC)
$ \begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c}$	Approaching vehice Direction of approa taffic: Right	cle count: 6 aching	
Motorcycle Pedestrian Tuk-Tuk Car Van Bicycle Light Truck Bus	O O dataset pSORT JiaPipe	Predictio	on of Pedestrian Intention

Muduli, K. and **Ghosh, I.** (2023) Prediction of the Future State of Pedestrians While Jaywalking Under Non-lanebased Heterogeneous Traffic Conditions. DOI: 10.1177/03611981231161619, *Transportation Research Record*, Sage Publications.





#### ADAS – higher Level of automation – I2V/I2P

Sequence based Prediction of Vehicular Yielding Intention





Muduli, K. and **Ghosh, I.** (2024) Prediction of Vehicular Yielding Intention While Approaching a Pedestrian Crosswalk. 10.1177/03611981241252835, **Transportation Research Record**, Sage Publications. 56



#### Predicting Pedestrian-Vehicle Interaction Severity at Unsignalized Intersections

Muduli, K., & Ghosh, I. (2025). Predicting Pedestrian-Vehicle Interaction Severity at Unsignalized Intersections. Traffic Injury Prevention. Taylor & Francis, (Accepted; Manuscript ID: 249718806)



#### BEHAVIOR OBSERVATION AND INTERACTION SEVERITY ESTIMATION

An AI-powered tool (DataFromSky Viewer) for analyzing trajectories, speeds, and acceleration of road users was used to estimate severity levels.

#### INTERACTION CATEGORIES

- Safe Passage: 1.
  - No imminent risk, smooth interactions, high PET (>3 sec).
  - Low speeds ( $\leq 10$  m/s), controlled trajectories.
  - Example: Vehicle yielding or negotiated passage.

#### Critical Event: 2.

- Moderate risk, closer temporal proximity, moderate PET (1-3 sec).
- Speeds (5-10 m/s), evasive maneuvers, moderate acceleration.
- Example: Pedestrian withdrawal or acceleration.

#### Conflict: 3.

- High risk, near-misses, low PET (<1 sec).
- High speeds (>10 m/s), emergency maneuvers.
- Example: Vehicle acceleration or emergency braking.

#### **OBSERVER TRAINING AND RELIABILITY**

6 observers evaluated 4,315 interactions. 

Inter-Rater Agreement:

- $\Box$  Measured using Fleiss Kappa ( $\kappa = 0.75$ )
- Demonstrated substantial agreement beyond chance

Muduli, K., & Ghosh, I. (2025). Predicting Pedestrian-Vehicle Interaction Severity at Unsignalized Intersections. Traffic Injury Prevention. Taylor & Francis, (Accepted; Manuscript ID: 249718806)



#### PROPOSED MODEL ARCHITECTURE AND TRAINING PROCESS

Muduli, K., & Ghosh, I. (2025). Predicting Pedestrian-Vehicle Interaction Severity at Unsignalized Intersections. Traffic Injury Prevention. Taylor & Francis, (Accepted; Manuscript ID: 249712806)

#### RESULTS

	Precision	Recall	F1-Score
Class 0 (Safe Passage)	0.78	0.91	0.84
Class 1 (Critical event)	0.93	0.85	0.89
Class 2 (Conflict)	0.95	0.86	0.90
Accuracy			0.87
Macro avg	0.89	0.87	0.88
Weighted avg	0.88	0.87	0.88





Muduli, K., & Ghosh, I. (2025). Predicting Pedestrian-Vehicle Interaction Severity at Unsignalized Intersections. Traffic Injury Prevention. Taylor & Francis, (Accepted; Manuscript ID: 249718806)

#### Safety evaluation at roundabout and unsignalized intersections





Conflict points and their types at (a) Site 1, (b) Site 2, and (c) Site 13







Different types of traffic conflicts at unsignalized junctions on NHs

Taylor & Francis And & Install Control

Check for updates

ARTICLE HISTORY

KEYWORDS

threshold; grash

Received 25 May 2017 Revised 10 September 2019

Accepted 16 September 2019

Unsignalized intersections;

post encroachment time:

#### Post encroachment time threshold identification for right-turn related crashes at unsignalized intersections on intercity highways under mixed traffic

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#### ABSTRACT

Unsignalized intersections in developing countries experience many serious conflicts between crosstraffic due to indiscipline traffic manoeuvrability. Historically, Post Encroachment Time (PET) has gained attention as a proximal indicator to analyze crossing conflicts. However, identifying an appropriate PET threshold to classify critical conflicts for highly heterogeneous traffic scenario is still an unexplored area. Consequently, this study proposes a novel approach of PET threshold identification with proof of application by collecting data from ten intersections located on four-lane intercity highways in the National Capital Region (NCR), India, Both crossing conflicts and right-turn related crash data (for the left-hand drive) are collected. Their correlations are thoroughly studied for each PET threshold using a quantitative technique considering all and individual vehicle categories. Finally, a qualitative analysis is done by ranking the sites based on cumulative PET and related crashes to verify the proposed quantitative technique. A PET threshold of 1s is obtained from both the techniques which can be used to identify critical conflicts for unsignalized intersections located on four-lane intercity highways. The proposed methodology will serve as an alternative, faster and effective tool to evaluate the hazardousness of unsignalized intersections located on intercity highways under highly heterogeneous traffic condition.

#### Introduction

In the developing economies like India, safety evaluation is a critical task due to highly heterogeneous traffic mix, traffic indiscipline, as well as the absence of standard guidelines for assessing the safety of various traffic facilities. These often lead to less consideration of safety aspects resulting into an excessive number of crashes. With about 1,50,000 deaths annually, India presently has the worst road traffic crash rate worldwide (MoRTH, 2017; WHO, 2018), Although, India is a signatory to Brasilia Declaration and committed to reducing the number of road accidents and fatalities by 50% at the end of this decade, unsignalized intersections are such locations which have consistently proved themselves very unsafe traffic facilities. As per the latest statistics, unsignalized intersections accounted for about 83% of the 1,75,853 crashes and 85% of 53,975 deaths that took place at intersection locations (MoRTH, 2017). These intersections see more traffic conflicts as traffic signals are not present and drivers of self-organizing traffic flow need to take appropriate decision for carrying out various maneuvers. Interestingly, for developing countries, these intersections behave quite differently in comparison to their western counterparts (Paul & Ghosh, 2018). The non-prioritized traffic, that is, minor road traffic, as well as right turning vehicles (for the left-hand drive) from the major road, do not yield to the right-of-way vehicles and undertake risky crossing or turning maneuvers by accepting smaller gaps in traffic conflict has been drawing enormous attention, as

between through traffic along major roads. The priorities are basically established by the situations drivers perceive (Ashalatha & Chandra, 2011; Patil & Pawar, 2014; Paul & Ghosh, 2018) and thus these junctions essentially function as uncontrolled ones where vehicles from all the directions attempt crossing and turning at the same time, increasing the probability of crashes. Hence, ensuring safety at such locations is really a critical and challenging task due to the complex nature of traffic.

In order to reduce crashes, safety evaluation has traditionally been carried out based upon police-reported crash data. Traffic crash data analysis can help to understand the overall trend of crash occurrence and identify the major contributory factors that can be valuable to implement relevant countermeasures. However, besides various shortcomings associated with conventional approach (i.e. long observation period, reactive nature, etc.), unavailability of crash data and improper information related to the crash pattern as well as location are quite common for developing countries. In order to address these issues, several researchers have advocated the use of indirect, short-term traffic safety measures which are 'proactive' in nature and being used as an alternative to historical crash data for the reliable and faster safety evaluation (Allen, Shin, & Cooper, 1978; Hayward, 1972; Parker & Zegger, 1989; Perkins & Bowman, 1986). Among various such surrogate road safety measures,

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Development of conflict severity index for safety evaluation of severe crash types at unsignalized intersections under mixed traffic

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ARTICLE INFO

Keywordz Critical conflict Severity Right-turn related crash Rear-end crash Unsignalised intero Intercity highways

#### ABSTRACT

Existing proximal indicators lack the capability of addressing the severe implications of a possible collision. This study proposes a conflict severity index (CSI) based on crash risk and expected collision severity for severe crash types at unsignalized intersections on intercity highways. CSI is estimated for critical crossing and rear-end conflicts observed at ten unsignalized junctions using Post Encroachment Time (PET), Delta t ( $\Delta t$ ) and the expected loss in kinetic energy (AKE). At is a newly proposed indicator reflecting the rear-end collision probability which is the difference between Modified Time to Collision (MTTC) and Time-to-Stop (Ts). To get critical crossing and rear-end conflicts, 'critical speed' and 1.5 s threshold value of  $\Delta t$  are used respectively. The study concept postulates that with the decreasing PET and  $\Delta t$  or an increasing  $\Delta RE$ , the CSI value increases. The sensitivity analytis shows that for a specific PET or  $\Delta t$  value, CSI increases with the increasing speed of right-ofway vehicle/follower, vehicular mass difference and conflict angle. For crossing conflicts, angle between involved vehicles plays a major role in increasing AKE and resulting CSI. Finally, relationships between CSI values of critical crossing/rear-end conflicts and related severe crash types (for left-hand traffic) demonstrate that the proposed index is an appropriate measure to reflect the probable collision severity. This index will be very useful in network screening for safety improvement and reducing the number of crash victims at hazardou locations.

1. Introduction

Reducing the severity of road crashes has been a critical concern for many years. Despite several studies aimed at identifying factors that affect crash injury sevenity, a thorough evaluation of sevenity consequences of potential collisions remains difficult using traditional crash data-based methods (Pai and Saleh, 2007; Pai 2009; Das and Abdel-Aty 2011: Arteaga et al., 2020). Safety research using crash data is logically rational and reliable, but it has some limitations such as i) wellrecognized availability and quality issues, ii) a longer observational period, iii) random and rare nature of crashes in a statistical sense, and iv) technique is reactive in nature (Chin and Quek, 1997; de Leur and Sayed (2003); Oh et al., 2010; El-Basyouny and Sayed, 2013; Sacchi and Sayed, 2016; Paul and Ghosh, 2018; Uzondu et al., 2018). These have led to an increased interest in safety diagnosis using traffic conflict technique (TCT) based on several proximal indicators for any roadway facility, including intersections (Sayed et al., 1994; Archer, 2005; Pin davani et al., 2010; Huang et al., 2013). TCT has several statistical

benefits, including a large sample size, short duration, and proactive nature (Amundsen and Hyden, 1977). At present, the majority of proximal safety indicators are utilized to detect traffic conflicts and determine the proximity of crash occurrence in terms of time and space. A few of them are Time to Accident (TA), Time to Collision (TTC), Post-Encroachment Time (PET), Deceleration Rate (DR), Proportion of stopping distance (PSD), etc. These indicators are limited to depict the severity consequences of possible collisions caused by vehicular interactions (Laureshyp et al. 2017). For example, a small TTC or PET value represents a minimal time to collide and a high propensity to convert a conflict in the collision. A similar TTC/PET value for various conflicts represents the same probability of collision occurrence. However, a crash involving small vehicles is very different from a collision resulting in injuries caused by buses or trucks, even though their TTC/ PET values are the same. Thus, the sevenity outcomes of potential collisions should be taken into account.

Few studies have explored the severity consequences of traffic conflicts. Hyden (1987) had first talked about the conflict severity in TCT

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#### Influence of green signal countdown timer on severe crash types at signalized intersections due to red light violations

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ABSTRACT

Traffic conflicts at signalized intersections caused by red light violations (RLVs) present an enormous danger for severe crashes. This study proactively evaluates the efficacy of Green Signal Countdown Timer (GSCT) on RLVs and resulting right-angle and right turn related crashes (for left-hand driving) using Post Encroachment Time (PET). A before-after (B-A) with comparison group analysis is done considering five 4-legged signalized intersections from Delhi, India comprising of with and without SCT installations. RLV occurrences and maximum violation time during initial 10 s of red are more in the absence of GSCT. Considering various RLV-related characteristics, Powered Two Wheelers (P2W) are found to have the highest exposure in crash involvement for both timer conditions. Finally, B-A method shows 57.74% reduction in crossing conflicts due to the presence of GSCT at the sites. Accordingly, SCT's installation can be recommended as a cost-effective engineering countermeasure to reduce severe crash types at signalized intersections.

#### Introduction

Red light violation (RLV) is one of the major factors contributing to motor vehicle crashes at urban signalized intersections. It takes places when a driver intentionally or unintentionally enters into the intersection from one of its approaches at the onset of red and involves in crash with another road user who has the right-of-way (Baratian-Ghorghi, Zhou, and Zech 2016). As per the recent statistics of the USA, an average of 719 people died each year between 2011 and 2015 in RLV-related crashes (American Traffic Solutions [ATSOL] 2017). In Thailand, 1702 red light running crashes at intersections were caused by drivers violating the red light (Kanitpong et al. 2015; Jensupakarn and Kanitpong 2018).

To reduce this non-compliant driving behavior and resulting crashes, several mitigation measures related to geometric changes, change in signal timings, installations of contemporary traffic control devices, e.g. red light cameras (RLCs) have been implemented during the last few decades. Although RLCs have increasingly been used to enforce violations at intersections and discourage a driver running the red light, their presence is found to be associated with a higher number of minor rear-end collisions (Council et al. 2005; Persaud et al. 2005; Baratian-Ghorghi, Zhou, and Zech 2016). In addition, RLCs are not always costeffectiveness considering their installations and maintenance (Chen and Warburton 2006).

The application of driver warning indicators in terms of distance and time reference aids is currently quite common to help drivers to make their decisions at signalized intersections. In recent years, a cost-effective technological implementation, namely, signal countdown timer (SCT), has become increasingly popular in numerous Asian including India, which operates as a time-warning system. Interestingly, this device was installed in India with the objective of saving valuable fossil fuel when motorists are stuck at the red light. However, their installations at major signalized intersections of several Indian cities have been taken up

without examining the safety aspects. SCT's green component, i.e. Green signal countdown timer (GSCT) is found to alter the driving behavior at the end of the green signal (Devalla, Biswas, and Ghosh 2015; Limanond, Prabjabok, and Tippayawong 2010; Chiou and Chang 2010; Fu et al. 2016; Biswas, Ghosh, and Chandra 2017). Simultaneously, its absence or presence may affect the occurrences of RLVs at the starting of red and thereby create the potentials for severe crash types. Among such crashes, the most severe ones are the right-angle and right turn related crashes (for left-hand drive) (Wang and Abdel-Aty 2007; Jin, Wang, and Chen 2010). In this context, the goal of the present research is to assess the safety impacts of GSCT on severe crash types at major urban signalized intersections. For this purpose, this study adopts the before-after (B-A) with comparison group study design based on Traffic Conflict Technique (TCT), which includes five signalized

intersections located in Delhi, consisting of both treatment and control sites (i.e. sites with and without SCT respectively). Of late, TCT has been drawing enormous attention owing to its proactive nature and several drawbacks associated with traditional crash data based assessment (Gettman and Head 2003; Zheng, Ismail, and Meng 2014; Sacchi and Sayed 2016; Paul and Ghosh 2018). TCT monitors the real-time traffic interactions even before the crashes occur and has the benefit of determining the propensity of resulting collisions at any road network including intersections. Its use for short term safety evaluation is well accepted worldwide, although TCT does not show the general trend of crashes and the associated contributory variables. Furthermore, for certain types of conflict, the crash conflict relationship is ambiguous and the correctness of the conversion to expected crash frequencies is doubtful. (Williams 1981: Laureshyn and Varhelvi 2018). Another significant point is that the police record does not always enable for the correct identification of severe RLV

crashes. There may be few crashes per year but hundreds of RLV

could have occurred at an intersection. Hence, it is essential to

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KEYWORDS Signalized intersection: red light violation; green signa countdown timer; beforeafter study; post encroachment time

> Kenwords Signalized intersection Dilemma zone Green signal countdown timer Yellow interval All red Microsimulation

During yellow intervals, dilemma zones (DZ) often trigger red light violations (RLV) and unexpected stopping near stop lines, causing severe collisions at signalized intersections. While several studies in developed countrie have tested various countermeasures to eliminate DZ, this aspect is not well explored for developing countries. This study investigated a green signal countdown timer's (GSCT) efficacy in reducing DZ at signalized intersections, Based on historical crash data, 10 signalized junctions in Delhi, India, 5 with GSCT and 5 without, were chosen. Although India has a permissive yellow law, all the study sites have a flat 5 s yellow change interval with no provision of all-red (AR) intervals, which might have resulted in severe crashes at these locations. Empirical assessments revealed that GSCT minimizes the length of type-I DZ, which refers to a situation when drivers can neither stop nor proceed to the intersection during the yellow signal. Interestingly, GSCT also minimizes the length of type-II DZ, which is an indecision zone based on drivers' 10% and 90% stopping probabilities. Summation of yellow and all-red intervals (Y + AR) obtained using Institute of Transportation Engineers (ITE) equations was found to be longer than field-allocated 5 s change interval at GSCT-enabled sites Consequently, GSCT's effectiveness across various vellow and all-red intervals was investigated in the PTV-VISSIM microsimulation tool, and crossing and rear-end conflicts were extracted using Surrogate Safety Assessment Model (SSAM), Results suggest that GSCT's presence, along with estimated vellow and all-red intervals, reduces crossing and rear-end conflicts due to RLVs and inconsistent stoppings, respectively.

#### 1. Introduction

Signalized intersections are critical locations of urban road networks which account for a significant number of severe road crashes. The vellow interval at signalized intersections often produces a dilemma zone (DZ) along the intersection approaches. Within DZ, a vehicle 'cannot cross' the entire intersection with its traveling speed without violating red light or 'cannot stop' safely with a comfortable deceleration (i.e., must stop suddenly). These often result in right-angle, right turn related crashes (for left-hand traffic) and rear-end collisions at signalized intersections. For the yellow interval, different countries have different legal requirements. In the nations where restrictive yellow law exists, ideally, only a yellow interval is given with no provision for an

all-red interval prohibiting the entry of drivers at an intersection. However, in regions where the yellow permissive law is followed, drivers can enter the intersection during a yellow light. In the case of permissive yellow law, it is also customary to have an all-red (AR) interval as it provides an extra buffer for vehicles to cross the intersections during the red time (NCUTLO, 1992)

In India, 9,712 crashes took place at signalized intersections in 2019. claiming 2.839 lives, of whom 541 died in crashes due to red light jumping (MoRTH, 2020). Besides red-light violations (RLVs), another prime reason for crashes is found to be sudden stoppings of vehicles along the intersection approaches. RLVs at the starting of the red often induce severe injury/fatal crashes between vehicles violating the red and vehicles from the conflicting traffic stream that recently received

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Safety Science

The effects of green signal countdown timer and retiming of signal intervals on dilemma zone related crash risk at signalized intersections under heterogeneous traffic conditions

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ABSTRACT

ARTICLE INFO



# **Credits: Amazing current doctoral students**



Anurag Thombre Two-wheeler Safety



Abhijnan Maji Roundabout Safety



Suvam Banerjee Emergency Evacuation @Cyclone



K Prathyusha NH Safety



Kaliprasana Muduli Unsignalized Pedestrian Crossing Safety



Avnish Panwar 3D modeling @Safety



Bhavna School Zone Safety



Sk Sahin Uddin Urban Road Safety



#### PhD ongoing – $\mathcal{9}$

numbers

- 1 PMRF
   4 single (Institute Assistantship)
- 3 Joint (Institute Assistantship)
- 1 Joint (Project)



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